

6.4.3

From Table 6.1: $A = by = 3(1.7) = 5.1 \text{ m}^2$; and

$$P = b + 2y = 3 + 2(1.7\text{m}) = 6.4 \text{ m.}$$

From Manning's eqn: $Q = (1/n)(A)^{5/3}(P)^{-2/3}(S)^{1/2}$

$$15 = (1/0.022)(5.1)^{5/3}(6.4)^{-2/3}(S)^{1/2}; S = \mathbf{0.00567}$$

From Eq'n 6.14: $y_c = [(Q^2/(gb^2))]^{1/3}$

$$y_c = [(15^2/(9.81 \cdot 3^2))]^{1/3} = \mathbf{1.37 \text{ m}}$$

from Eq'n 6.12: $N_f = V/(gD)^{1/2} = (Q/A)/(gD)^{1/2}$

$$N_f = (15/5.1)/(9.81 \cdot 1.7)^{1/2} = \mathbf{0.720}$$

$N_f < 1$, therefore flow is **subcritical**.

6.4.6

From Table 6.1: $A = (\pi/8)d_o^2 = (\pi/8)(2.25)^2 = 1.99 \text{ ft}^2$

$$P = (\pi/2)d_o = (\pi/2)(2.25) = 3.53 \text{ ft}; R_h = A/P = 0.564 \text{ ft};$$

From the Manning eq'n: $V = (1.49/n)(R_h)^{2/3}(S)^{1/2}$

$$V = (1.49/0.024)(0.564)^{2/3}(0.005)^{1/2} = 3.00 \text{ ft/s}$$

$$E = V^2/(2g) + y = (3.00)^2/(2 \cdot 32.2) + 1.13 = \mathbf{1.27 \text{ ft}};$$

$$N_f = V/(gD)^{1/2}; D = A/T = 1.99/2.25 = 0.884;$$

$$N_f = (3.00)/(32.2 \cdot 0.884)^{1/2} = 0.562 (< \mathbf{1, \text{subcritical}})$$

6.4.7

$N_f = V/(gD)^{1/2}$; From Table 6.1: $A = (b + my)y$

$$A = [10\text{ft} + 1(8.3\text{ft})](8.3\text{ft}) = 152 \text{ ft}^2;$$

$$T = b + 2my = 10\text{ft} + 2(1)(8.3\text{ft}) = 26.6 \text{ ft}$$

$$V = Q/A = 3.16 \text{ ft/s}; \quad D = A/T = 152/26.6 = 5.71 \text{ ft}$$

$$N_f = V/(gD)^{1/2} = 3.16/(32.2 \cdot 5.71)^{1/2} = \mathbf{0.233}$$

(subcritical)

$$E = V^2/2g + y = (3.16)^2/(2 \cdot 32.2) + 8.3 = \mathbf{8.46 \text{ ft}}$$

For critical depth using Figure 6.9a:

$$(Qm^{3/2})/(g^{1/2}b^{5/2}) = [(480)1^{3/2}]/(32.2^{1/2}10^{5/2}) = 0.267;$$

$$\text{From Fig 6.9a, } my_c/b = (1)y_c/10 = 0.36; \quad y_c = \mathbf{3.60 \text{ ft}}$$

Alternatively use Eq'n 6.13; $Q^2/g = A^3/T$;

This equation yields: $y_c = \mathbf{3.65 \text{ ft}}$

6.4.8

Using Internet freeware (open channel calculators):

$$y_n = 6.92 \text{ ft} \quad \text{and} \quad y_c = 3.90 \text{ ft}$$

To determine a specific energy curve:

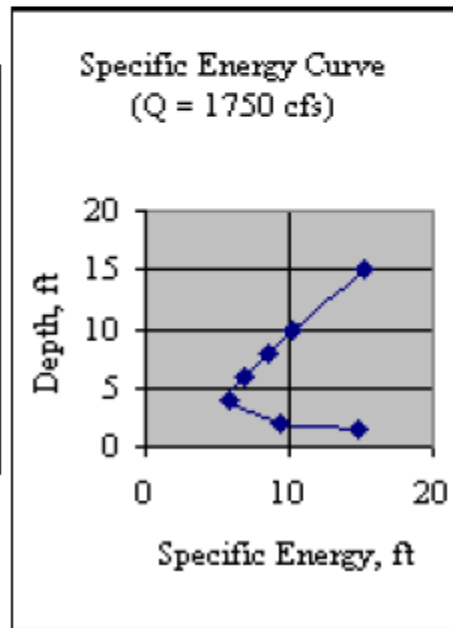
$$E = Q^2/(2gA^2) + y = (1750)^2/[2g\{(40)(y)\}^2] + y$$

Substituting different y 's on either side of $y_c = 3.90 \text{ ft}$

$$Q = 1750 \quad \text{cfs}$$

$$b = 40.0 \quad \text{ft}$$

| y (ft) | E (ft) |
|-------------|-------------|
| 1.5 | 14.7 |
| 2.0 | 9.4 |
| 3.9 | 5.9 |
| 6.0 | 6.8 |
| 8.0 | 8.5 |
| 10.0 | 10.3 |
| 15.0 | 15.1 |



6.4.10

Using Internet freeware (open channel calculators):

At the entrance to the transition, when $b = 12$ ft:

$y_i = 5.88$ ft; $V_i = 7.09$ ft/sec; and at the transition exit,

when $b = 6$ ft: $y_e = 13.4$ ft; $V_e = 6.23$ ft/sec

Based on an energy balance: $H_i = H_e + h_L$ or

$V_i^2/2g + y_i + z_i = V_e^2/2g + y_e + z_e + h_L$; letting $z_e = 0$

$(7.09)^2/2g + 5.88$ ft + $z_i = (6.23)^2/2g + 13.4$ ft + 0 + 1.5 ft

$z_i = 8.84$ ft (the channel bottom height at the inlet)

The table below provides the transition properties.

$H = H_i - h_L$; (losses uniformly distributed in transition)

$E =$ specific energy = $H - z$, and y is found from

$$E = V^2/2g + y = Q^2/[2g\{(b)(y)\}^2] + y$$

| Section | Width, b (ft) | z (ft) | H (ft) | E (ft) | y (ft) |
|---------|------------------|-----------|-----------|-----------|-----------|
| Inlet | 12.0 | 8.84 | 15.50 | 6.66 | 5.88 |
| 25 | 10.5 | 6.63 | 15.13 | 8.50 | 7.94 |
| 50 | 9.0 | 4.42 | 14.75 | 10.33 | 9.80 |
| 75 | 7.5 | 2.21 | 14.38 | 12.17 | 11.70 |
| Exit | 6.0 | 0.00 | 14.00 | 14.00 | 13.40 |

